

CLEAN ENERGY PROJECT ANALYSIS: RETScreen® ENGINEERING & CASES TEXTBOOK



CANMET Energy Technology
Centre - Varennes (CETC)

In collaboration with:



SMALL HYDRO PROJECT ANALYSIS CHAPTER

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SMALL HYDRO PROJECT ANALYSIS CHAPTER

Clean Energy Project Analysis: RETScreen® Engineering & Cases is an electronic textbook for professionals and university students. This chapter covers the analysis of potential small hydro projects using the RETScreen® International Clean Energy Project Analysis Software, including a technology background and a detailed description of the algorithms found in the RETScreen® Software. A collection of project case studies, with assignments, worked-out solutions and information about how the projects fared in the real world, is available at the RETScreen® International Clean Energy Decision Support Centre Website www.retscreen.net.

1 SMALL HYDRO BACKGROUND¹

Hydroelectricity is one of the most mature forms of renewable energy, providing more than 19% of the world's electricity consumption from both large and small power plants. Countries such as Brazil, the United States, Canada and Norway produce significant amounts of electricity from very large hydroelectric facilities. However, there are also many regions of the world that have a significant number of small hydro power plants in operation, such as the one depicted in **Figure 1**. In China, for example, more than 19,000 MW of electricity is produced from 43,000 small hydro facilities.

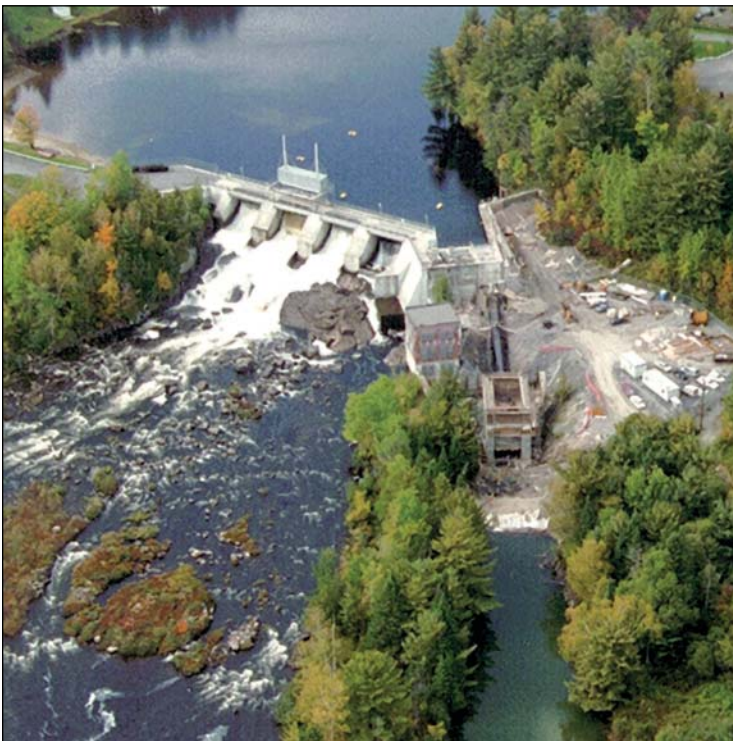


Figure 1:

2.6 MW Small Hydro Power
Project in Canada.

Photo Credit:
SNC-Lavalin

1. Some of the text in this "Background" description comes from the following reference: Bennett, K., **Small Hydro in Canada: An Overview**, prepared for Industry, Science and Technology Canada, Aboriginal Economic Programs, 1990.



There is no universally accepted definition of the term “small hydro” which, depending on local definitions can range in size from a few kilowatts to 50 megawatts or more of rated power output. Internationally, “small” hydro power plant capacities typically range in size from 1 MW to 50 MW, with projects in the 100 kW to 1 MW range sometimes referred to as “mini” hydro and projects under 100 kW referred to as “micro” hydro. Installed capacity, however, is not always a good indicator of the size of a project. For example, a 20 MW, low-head “small” hydro plant is anything but small as low-head projects generally use much larger volumes of water, and require larger turbines as compared with high-head projects.

1.1 Description of Small Hydro Power Plants

A small hydro generating station can be described under two main headings: civil works, and electrical and mechanical equipment. Refer to *Figure 2* below for a schematic of a typical small hydro power plant.

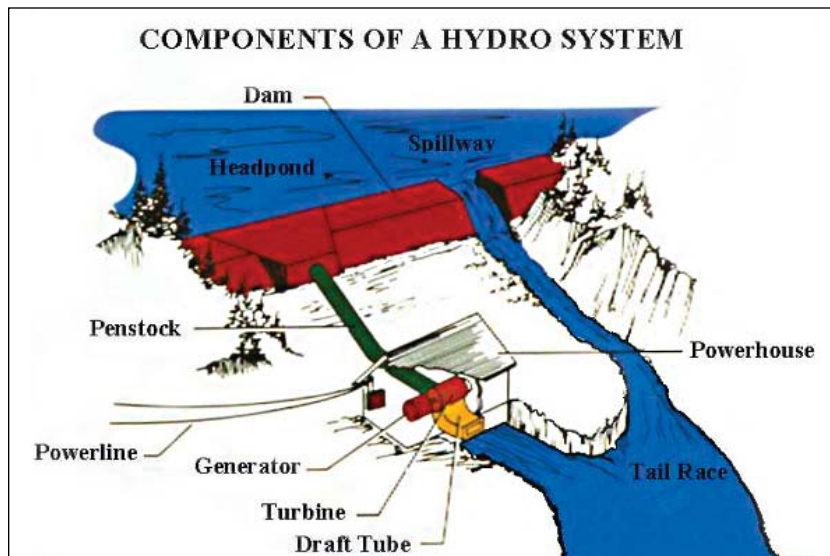


Figure 2:
*Small Hydro Project
System Schematic.*

1.1.1 Civil works

The main civil works of a small hydro development are the diversion dam or weir, the water passages and the powerhouse as depicted in *Figure 3*. The diversion dam or weir directs the water into a canal, tunnel, penstock or turbine inlet. The water then passes through the turbine, spinning it with enough force to create electricity in a generator. The water then flows back into the river via a tailrace. Generally, small hydro projects built for application at an isolated area are run-of-river developments, meaning that water is not stored in a reservoir and is used only as it is available. The cost of large water storage dams cannot normally be justified for small waterpower projects and consequently, a low dam or diversion weir of the simplest construction is normally used. Construction can be of concrete, wood, masonry or a combination of these materials. Considerable effort continues to be spent to lower the cost of dams and weirs for small hydro projects, as the cost of this item alone frequently renders a project not financially viable.



Figure 3:

Civil Works for a 700 kW Mini Hydro Project.

Photo Credit:

Ottawa Engineering

The water passages of a small hydro project comprise the following:

- An intake which includes trashracks, a gate and an entrance to a canal, penstock or directly to the turbine depending on the type of development. The intake is generally built of reinforced concrete, the trashrack of steel, and the gate of wood or steel.
- A canal, tunnel and/or penstock, which carries the water to the powerhouse in developments where the powerhouse is located at a distance downstream from the intake. Canals are generally excavated and follow the contours of the existing terrain. Tunnels are underground and excavated by drilling and blasting or by using a tunnel-boring machine. Penstocks, which convey water under pressure, can be made of steel, iron, fibreglass, plastics, concrete or wood.
- The entrance and exit of the turbine, which include the valves and gates necessary to shut off flow to the turbine for shutdown and maintenance. These components are generally made of steel or iron. Gates downstream of the turbine, if required for maintenance, can be made of wood.



- A tailrace, which carries the water from the turbine exit back to the river. The tailrace, like the canal, is excavated.

The powerhouse contains the turbine or turbines and most of the mechanical and electrical equipment as depicted in **Figure 4**. Small hydro powerhouses are generally kept to the minimum size possible while still providing adequate foundation strength, access for maintenance, and safety. Construction is of concrete and other local building materials.

Simplicity in design, with an emphasis on practical, easily constructed civil structures is of prime concern for a small hydro project in order to keep costs at a minimum.

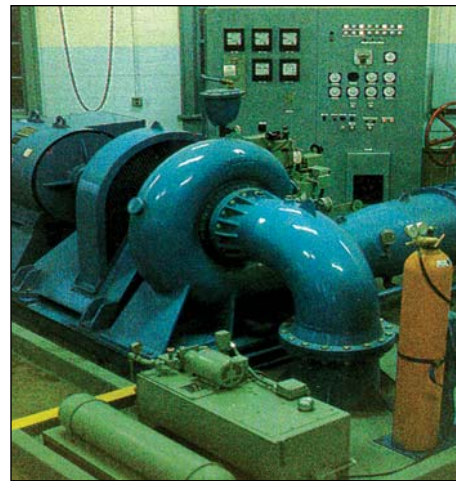


Figure 4:
Small Hydro Powerhouse Containing Francis Turbine.

Photo Credit:
PO Sjöman Hydrotech Consulting

1.1.2 Electrical and mechanical equipment

The primary electrical and mechanical components of a small hydro plant are the turbine(s) and generator(s).

A number of different types of turbines have been designed to cover the broad range of hydropower site conditions found around the world. Turbines used for small hydro applications are scaled-down versions of turbines used in conventional large hydro developments.

Turbines used for low to medium head applications are usually of the reaction type and include Francis and fixed and variable pitch (Kaplan) propeller turbines. The runner or turbine “wheel” of a reaction turbine is completely submersed in water. Turbines used for high-head applications are generally referred to as impulse turbines. Impulse turbines include the Pelton (see **Figure 5**), Turgo and crossflow designs. The runner of an impulse turbine spins in the air and is driven by a high-speed jet of water.

Small hydro turbines can attain efficiencies of about 90%. Care must be given to selecting the preferred turbine design for each application as some turbines only operate efficiently over a limited flow range (e.g. propeller turbines with fixed blades and Francis turbines). For most run-of-river small hydro sites where flows vary considerably, turbines that operate efficiently over a wide flow range are usually preferred (e.g. Kaplan, Pelton, Turgo and crossflow designs). Alternatively, multiple turbines that operate within limited flow ranges can be used.

There are two basic types of generators used in small hydro plants - synchronous or induction (asynchronous). A synchronous generator can be operated in isolation while an induction generator must normally be operated in conjunction with other generators. Synchronous generators are used as the primary source of power produced by utilities and for isolated diesel-grid and stand-alone small hydro applications. Induction generators with



capacities less than about 500 kW are generally best suited for small hydro plants providing energy to a large existing electricity grid.

Other mechanical and electrical components of a small hydro plant include:

- Speed increaser to match the ideal rotational speed of the turbine to that of the generator (if required);
- Water shut-off valve(s) for the turbine(s);
- River by-pass gate and controls (if required);
- Hydraulic control system for the turbine(s) and valve(s);
- Electrical protection and control system;
- Electrical switchgear;
- Transformers for station service and power transmission;
- Station service including lighting and heating and power to run control systems and switchgear;
- Water cooling and lubricating system (if required);
- Ventilation system;
- Backup power supply;
- Telecommunication system;
- Fire and security alarm systems (if required); and
- Utility interconnection or transmission and distribution system.

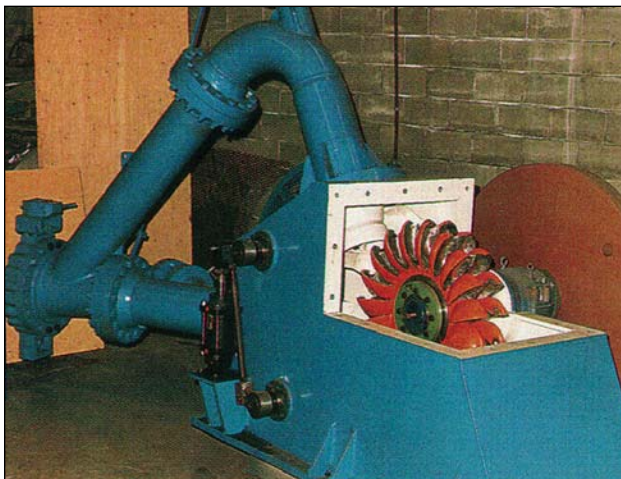


Figure 5:
Pelton Turbine.

Photo Credit:
PO Sjöman Hydrotech Consulting



1.2 Small Hydro Project Development

The development of small hydro projects typically takes from 2 to 5 years to complete, from conception to final commissioning. This time is required to undertake studies and design work, to receive the necessary approvals and to construct the project. Once constructed, small hydro plants require little maintenance over their useful life, which can be well over 50 years. Normally, one part-time operator can easily handle operation and routine maintenance of a small hydro plant, with periodic maintenance of the larger components of a plant usually requiring help from outside contractors.

The technical and financial viability of each potential small hydro project are very site specific. Power output depends on the available water (flow) and head (drop in elevation). The amount of energy that can be generated depends on the quantity of water available and the variability of flow throughout the year. The economics of a site depends on the power (capacity) and the energy that a project can produce, whether or not the energy can be sold, and the price paid for the energy. In an isolated area (off-grid and isolated-grid applications) the value of energy generated for consumption is generally significantly more than for systems that are connected to a central-grid. However, isolated areas may not be able to use all the available energy from the small hydro plant and, may be unable to use the energy when it is available because of seasonal variations in water flow and energy consumption.

RETScreen® International Small Hydro Project Model

The RETScreen® International Small Hydro Project Model can be used world-wide to easily evaluate the energy production, life-cycle costs and greenhouse gas emissions reduction for central-grid, isolated-grid and off-grid small hydro projects, ranging in size from multi-turbine small and mini hydro installations to single-turbine micro hydro systems.

A conservative, “rule-of-thumb” relationship is that power for a hydro project is equal to seven times the product of the flow (Q) and gross head (H) at the site ($P = 7QH$). Producing 1 kW of power at a site with 100 m of head will require one-tenth the flow of water that a site with 10 m of head would require. The hydro turbine size depends primarily on the flow of water it has to accommodate. Thus, the generating equipment for higher-head, lower-flow installations is generally less expensive than for lower-head, higher-flow plants. The same cannot necessarily be said for the civil works components of a project which are related much more to the local topography and physical nature of a site.





1.2.1 Types of small hydro developments

Small hydro projects can generally be categorised as either “run-of-river developments” or “water storage (reservoir) developments,” which are described in more detail below.

■ Run-of-river developments

“Run-of-river” refers to a mode of operation in which the hydro plant uses only the water that is available in the natural flow of the river, as depicted in **Figure 6**. “Run-of-river” implies that there is no water storage and that power fluctuates with the stream flow.



Figure 6:

Run-of-River Small Hydro Project in a Remote Community.

Photo Credit:

Robin Hughes/PNS

The power output of run-of-river small hydro plants fluctuates with the hydrologic cycle, so they are often best suited to provide energy to a larger electricity system. Individually, they do not generally provide much firm capacity. Therefore, isolated areas that use small hydro resources often require supplemental power. A run-of-river plant can only supply all of the electrical needs of an isolated area or industry if the minimum flow in the river is sufficient to meet the load’s peak power requirements.

Run-of-river small hydro can involve diversion of the flow in a river. Diversion is often required to take advantage of the drop in elevation that occurs over a distance in the river. Diversion projects reduce the flow in the river between the intake and the powerhouse. A diversion weir or small dam is usually required to divert the flow into the intake.



■ **Water storage (reservoir) developments**

For a hydroelectric plant to provide power on demand, either to meet a fluctuating load or to provide peak power, water must be stored in one or more reservoirs². Unless a natural lake can be tapped, providing storage usually requires the construction of a dam or dams and the creation of new lakes. This impacts the local environment in both negative and positive ways, although the scale of development often magnifies the negative impacts. This often presents a conflict, as larger hydro projects are attractive because they can provide “stored” power during peak demand periods. Due to the economies of scale and the complex approval process, storage schemes tend to be relatively large in size.

The creation of new storage reservoirs for small hydro plants is generally not financially viable except, possibly, at isolated locations where the value of energy is very high. Storage at a small hydro plant, if any, is generally limited to small volumes of water in a new head pond or existing lake upstream of an existing dam. Pondage is the term used to describe small volumes of water storage. Pondage can provide benefits to small hydro plants in the form of increased energy production and/or increased revenue.

Another type of water storage development is “pumped storage” where water is “recycled” between downstream and upstream storage reservoirs. Water is passed through turbines to generate power during peak periods and pumped back to the upper reservoir during off-peak periods. The economics of pumped storage projects depends on the difference between the values of peak and off-peak power. Due to the inefficiencies involved in pumping versus generating, the recycling of water results in a net consumption of energy. Energy used to pump water has to be generated by other sources.

The environmental impacts that can be associated with small hydro developments can vary significantly depending on the location and configuration of the project.

The effects on the environment of developing a run-of-river small hydro plant at an existing dam are generally minor and similar to those related to the expansion of an existing facility. Development of a run-of-river small hydro plant at an undeveloped site can pose additional environmental impacts. A small dam or diversion weir is usually required. The most economical development scheme might involve flooding some rapids upstream of the new small dam or weir.

The environmental impacts that can be associated with hydroelectric developments that incorporate water storage (typically larger in size) are mainly related to the creation of a water storage reservoir. The creation of a reservoir involves the construction of a relatively large dam, or the use of an existing lake to impound water. The creation of a new reservoir with a dam involves the flooding of land upstream of the dam. The use of water stored in the reservoir behind a dam or in a lake results in the fluctuation of water levels and flows in the river downstream. A rigorous environmental assessment is typically required for any project involving water storage.

2. Except in the run-of-river case where the minimum flow in the river can provide the peak power requirement.





1.2.2 Hydro project engineering phases

According to Gordon (1989), there are normally four phases for engineering work required to develop a hydro project. Note, however, that for small hydro, the engineering work is often reduced to three phases in order to reduce costs. Generally, a preliminary investigation is undertaken that combines the work involved in the first two phases described below. The work, however, is completed to a lower level of detail in order to reduce costs. While reducing the engineering work increases the risk of the project not being financially viable, this can usually be justified due to the lower costs associated with smaller projects.

■ *Reconnaissance surveys and hydraulic studies*

This first phase of work frequently covers numerous sites and includes: map studies; delineation of the drainage basins; preliminary estimates of flow and floods; and a one day site visit to each site (by a design engineer and geologist or geotechnical engineer); preliminary layout; cost estimates (based on formulae or computer data); a final ranking of sites based on power potential; and an index of cost.

■ *Pre-feasibility study*

Work on the selected site or sites would include: site mapping and geological investigations (with drilling confined to areas where foundation uncertainty would have a major effect on costs); a reconnaissance for suitable borrow areas (e.g. for sand and gravel); a preliminary layout based on materials known to be available; preliminary selection of the main project characteristics (installed capacity, type of development, etc.); a cost estimate based on major quantities; the identification of possible environmental impacts; and production of a single volume report on each site.

■ *Feasibility study*

Work would continue on the selected site with a major foundation investigation programme; delineation and testing of all borrow areas; estimation of diversion, design and probable maximum floods; determination of power potential for a range of dam heights and installed capacities for project optimisation; determination of the project design earthquake and the maximum credible earthquake; design of all structures in sufficient detail to obtain quantities for all items contributing more than about 10% to the cost of individual structures; determination of the dewatering sequence and project schedule; optimisation of the project layout, water levels and components; production of a detailed cost estimate; and finally, an economic and financial evaluation of the project including an assessment of the impact on the existing electrical grid along with a multi-volume comprehensive feasibility report.

■ *System planning and project engineering*

This work would include studies and final design of the transmission system; integration of the transmission system; integration of the project into the power network to determine precise operating mode; production of tender drawings and specifications; analysis of bids and detailed design of the project; production of detailed construction drawings and review of manufacturer's equipment drawings. However, the scope of this phase would not include site supervision nor project management, since this work would form part of the project execution costs.





2 RETSCREEN SMALL HYDRO PROJECT MODEL

The RETScreen Small Hydro Project Model provides a means to assess the available energy at a potential small hydro site that could be provided to a central-grid or, for isolated loads, the portion of this available energy that could be harnessed by a local electric utility (or used by the load in an off-grid system). The model addresses both run-of-river and reservoir developments, and it incorporates sophisticated formulae for calculating efficiencies of a wide variety of hydro turbines.

The Small Hydro model can be used to evaluate small hydro projects typically classified under the following three names:

- Small hydro;
- Mini hydro; and
- Micro hydro.

The Small Hydro Project Model has been developed primarily to determine whether work on the small hydro project should proceed further or be dropped in favour of other alternatives. Each hydro site is unique, since about 75% of the development cost is determined by the location and site conditions. Only about 25% of the cost is relatively fixed, being the cost of manufacturing the electromechanical equipment.

Seven worksheets (*Energy Model*, *Hydrology Analysis and Load Calculation (Hydrology & Load)*, *Equipment Data*, *Cost Analysis*, *Greenhouse Gas Emission Reduction Analysis (GHG Analysis)*, *Financial Summary* and *Sensitivity and Risk Analysis (Sensitivity)*) are provided in the Small Hydro Project Workbook file.

The **Energy Model**, **Hydrology & Load** and **Equipment Data** worksheets are completed first. The **Cost Analysis** worksheet should then be completed, followed by the **Financial Summary** worksheet. The **GHG Analysis** and **Sensitivity** worksheets are optional analysis. The *GHG Analysis* worksheet is provided to help the user estimate the greenhouse gas (GHG) mitigation potential of the proposed project. The *Sensitivity* worksheet is provided to help the user estimate the sensitivity of important financial indicators in relation to key technical and financial parameters. In general, the user works from top-down for each of the worksheets. This process can be repeated several times in order to help optimise the design of the small hydro project from an energy use and cost standpoint.

The RETScreen Small Hydro Project Model provides the user with two different methods for estimating project costs: the “Formula” and the “Detailed” costing methods. All the hydro cost equations used in the “Formula” costing method are empirical, based on data collected over 20 years for both large and small hydro facilities. They have been extended to include more site data for this analysis (Gordon, 1989 & 1991). If used correctly, the “Formula” costing method will provide a baseline, or minimum, cost estimate for a proposed project.





The “Detailed” costing method allows the user to estimate costs based on estimated quantities and unit costs. The use of this costing method requires that the user estimate the size and the layout of the required structures. If the user chooses to use this method, the results should be compared with results from the “Formula” costing method.

In order to use the RETScreen Small Hydro Project Model, the user may require certain information that can be obtained from available topographic maps. Topographic maps can be purchased or ordered from most map stores. In cases where a previous hydrologic assessment has been undertaken for the site in question, the pertinent data from this assessment can be used in the model. The user should be aware that if the available head, or drop in elevation, at a site is unknown, a site visit will be required to measure the head unless detailed mapping is available. The measurement of head can be done easily using simple surveying techniques.

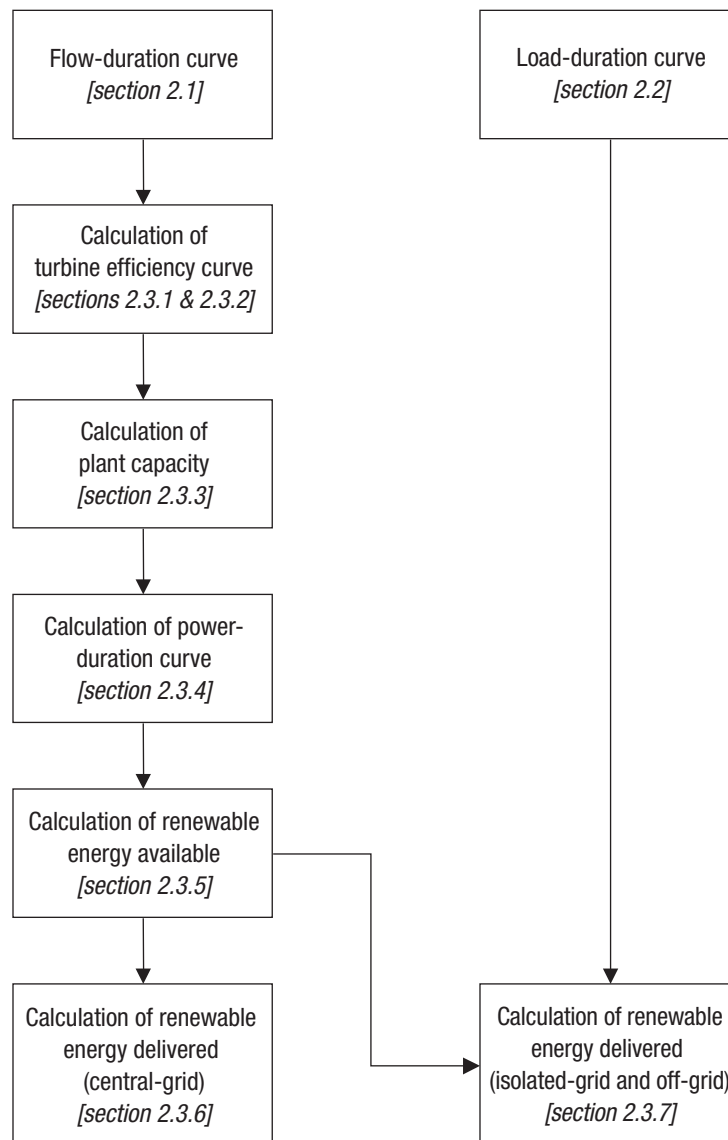
This section describes the various algorithms used to calculate, on an annual basis, the energy production of small hydro power plants in RETScreen. A flowchart of the algorithms is shown in **Figure 7**. User inputs include the flow-duration curve (*Section 2.1*) and, for isolated-grid and off-grid applications, the load-duration curve (*Section 2.2*). Turbine efficiency is calculated at regular intervals on the flow-duration curve (*Sections 2.3.1 and 2.3.2 and Appendix A*). Plant capacity is then calculated (*Section 2.3.3*) and the power-duration curve is established (*Section 2.3.4*). Available energy is simply calculated by integrating the power-duration curve (*Section 2.3.5*). In the case of a central-grid, the energy delivered is equal to the energy available (*Section 2.3.6*). In the case of an isolated-grid or off-grid application, the procedure is slightly more complicated and involves both the power-duration curve and the load-duration curve (*Section 2.3.7*). The Formula Costing Method (*Section 2.4*) is described in detail in *Appendix B* and a validation of the RETScreen Small Hydro Project Model is presented in *Section 2.5*.

There are some limitations associated with the Small Hydro Project Model. First, the model has been designed primarily to evaluate run-of-river small hydro projects. The evaluation of storage projects is possible, however, a number of assumptions are required. Variations in gross head due to changes in reservoir water level cannot be simulated. The model requires a single value for gross head and, in the case of reservoir projects, an appropriate average value must be entered. The determination of the average head must be done outside of the model and will require an understanding of the effects of variations in head on annual energy production. Second, for isolated-grid and off-grid applications in isolated areas, the energy demand has been assumed to follow the same pattern for every day of the year. For isolated locations where energy demand and available energy vary significantly over the course of a year, adjustments will have to be made to the estimated amount of renewable energy delivered. This is done by changing the “Available flow adjustment factor” in the *Energy Model* worksheet. These limitations aside, the model is fairly easy to understand and use. As will be seen in the next sections, the model condenses in an easy-to-use format a wealth of information, and it should be of great assistance to engineers involved in the preliminary evaluation of small hydro projects.





Figure 7:
Small Hydro Energy
Model Flowchart.



2.1 Hydrology

In RETScreen, hydrological data are specified as a flow-duration curve, which is assumed to represent the flow conditions in the river being studied over the course of an average year. For storage projects, data must be entered manually by the user and should represent the regulated flow that results from operating a reservoir; at present, the head variation with storage drawdown is not included in the model. For run-of-river projects, the required flow-duration curve data can be entered either manually or by using the specific run-off method and data contained in the RETScreen Online Weather Database.



A flow-duration curve is a graph of the historical flow at a site ordered from maximum to minimum flow. The flow-duration curve is used to assess the anticipated availability of flow over time, and consequently the power and energy, at a site. The model then calculates the firm flow that will be available for electricity production based on the flow-duration curve data, the percent time the firm flow should be available and the residual flow.

2.1.1 Flow-duration curve

The flow-duration curve is specified by twenty-one values Q_0, Q_5, \dots, Q_{100} representing the flow on the flow-duration curve in 5% increments. In other words, Q_n represents the flow that is equalled or exceeded $n\%$ of the time. An example of a flow-duration curve is shown in **Figure 8**.

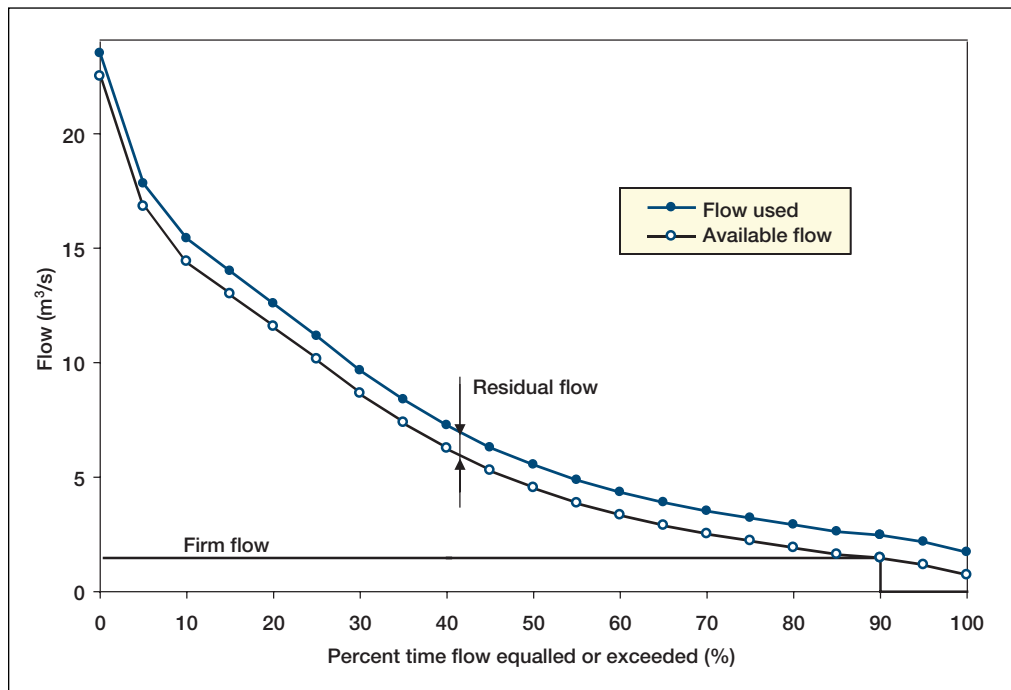


Figure 8:
Example of a Flow-Duration Curve.

When the specific run-off method is used, the flow-duration curve is expressed in normalised form, i.e. relative to the mean flow. The mean flow \bar{Q} is calculated as:

$$\bar{Q} = R A_D \tag{1}$$



where R is the specific run-off and A_D is the drainage area. Then the actual flow data Q_n ($n=0,5,\dots,100$) is computed from the normalised flow data q_n extracted from the weather database through:

$$Q_n = q_n \bar{Q} \quad (2)$$

2.1.2 Available flow

Often, a certain amount of flow must be left in the river throughout the year for environmental reasons. This *residual flow* Q_r is specified by the user and must be subtracted from all values of the flow-duration curve for the calculation of plant capacity, firm capacity and renewable energy available, as explained further on in this chapter. The *available flow* Q'_n ($n = 0, 5, \dots, 100$) is then defined by:

$$Q'_n = \max(Q_n - Q_r, 0) \quad (3)$$

The available flow-duration curve is shown in **Figure 8**, with as an example Q_r set to 1 m³/s.

2.1.3 Firm flow

The firm flow is defined as the flow being available $p\%$ of the time, where p is a percentage specified by the user and usually equal to 95%. The firm flow is calculated from the available flow-duration curve. If necessary, a linear interpolation between 5% intervals is used to find the firm flow. In the example of **Figure 8** the firm flow is equal to 1.5 m³/s with p set to 90%.

2.2 Load

The degree of sophistication used to describe the load depends on the type of grid considered. If the small hydro power plant is connected to a central-grid, then it is assumed that the grid absorbs all of the energy production and the load does not need to be specified. If on the other hand the system is off-grid or connected to an isolated-grid, then the portion of the energy that can be delivered depends on the load. The RETScreen Small Hydro Project Model assumes that the daily load demand is the same for all days of the year and can be represented by a load-duration curve. An example of such a curve is shown in **Figure 9**. As for the flow-duration curve of **Section 2.1.1**, the load-duration curve is specified by twenty-one values L_0, L_5, \dots, L_{100} defining the load on the load-duration curve in 5% increments: L_k represents the load that is equalled or exceeded $k\%$ of the time.



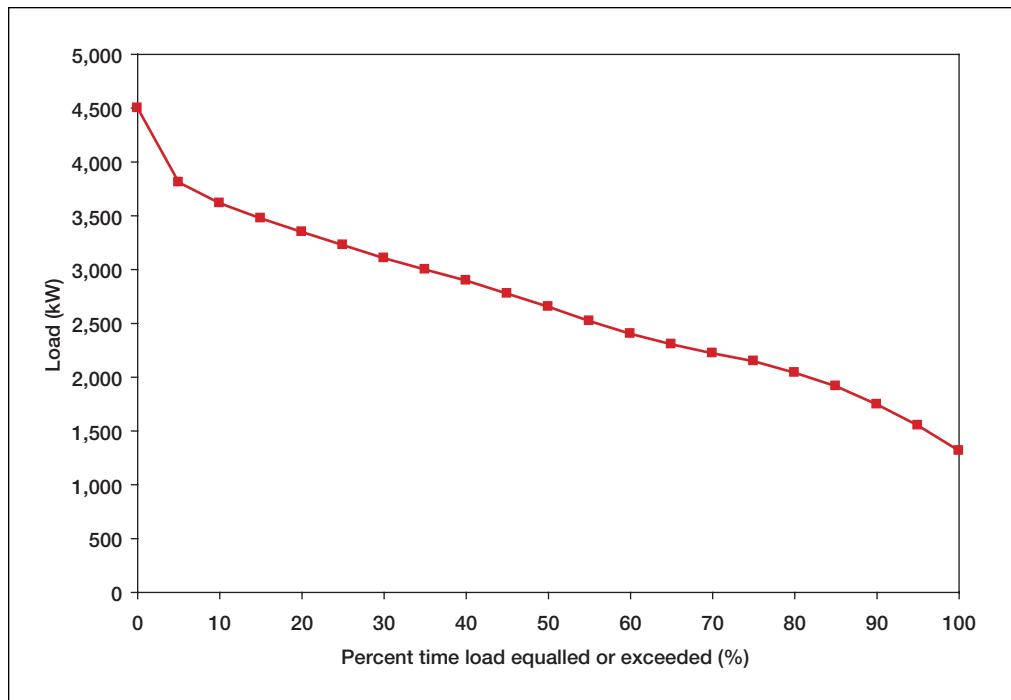


Figure 9:
Example of a Load-Duration Curve.

2.2.1 Energy demand

Daily energy demand³ is calculated by integrating the area under the load-duration curve over one day. A simple trapezoidal integration formula is used. The daily demand D_d expressed in kWh is therefore calculated as:

$$D_d = \sum_{k=1}^{20} \left(\frac{L_{5(k-1)} + L_{5k}}{2} \right) \frac{5}{100} 24 \quad (4)$$

with the L expressed in kW. The annual energy demand D is obtained by multiplying the daily demand by the number of days in a year, 365:

$$D = 365 D_d \quad (5)$$

3. It is assumed that the reader is already familiar with the concepts of **load** and **demand**. Load refers to instantaneous values (power, expressed for example in W) whereas demand refers to integrated values (energy, expressed for example in J or in Wh).





2.2.2 Average load factor

The average load factor \bar{L} is the ratio of the average daily load ($D_d/24$) to the peak load (L_0):

$$\bar{L} = \frac{D_d/24}{L_0} \quad (6)$$

This quantity is not used by the rest of the algorithm but is simply provided to the user to give an indication of the variability of the load.

2.3 Energy Production

The RETScreen Small Hydro Project Model calculates the estimated renewable energy delivered (MWh) based on the adjusted available flow (adjusted flow-duration curve), the design flow, the residual flow, the load (load-duration curve), the gross head and the efficiencies/losses. The calculation involves comparing the daily renewable energy available to the daily load-duration curve for each of the flow-duration curve values.

2.3.1 Turbine efficiency curve

Small hydro turbine efficiency data can be entered manually or can be calculated by RETScreen. Calculated efficiencies can be adjusted using the *Turbine manufacture/design coefficient* and *Efficiency adjustment* factor in the *Equipment Data* worksheet of the model. Standard turbine efficiencies curves have been developed for the following turbine types:

- Kaplan (reaction turbine)
- Francis (reaction turbine)
- Propellor (reaction turbine)
- Pelton (impulse turbine)
- Turgo (impulse turbine)
- Cross-flow (generally classified as an impulse turbine).

The type of turbine is selected based on its suitability to the available head and flow conditions. The calculated turbine efficiency curves take into account a number of factors including rated head (gross head less maximum hydraulic losses), runner diameter (calculated), turbine specific speed (calculated for reaction turbines) and the turbine manufacture/design coefficient. The efficiency equations were derived from a large number of manufacture efficiency curves for different turbine types and head and flow conditions. The turbine efficiency equations are described in **Appendix A**.



For multiple turbine applications it is assumed that all turbines are identical and that a single turbine will be used up to its maximum flow and then flow will be divided equally between two turbines, and so on up to the maximum number of turbines selected. The turbine efficiency equations and the number of turbines are used to calculate plant turbine efficiency from 0% to 100% of design flow (maximum plant flow) at 5% intervals. An example turbine efficiency curve is shown in **Figure 10** for 1 and 2 turbines.

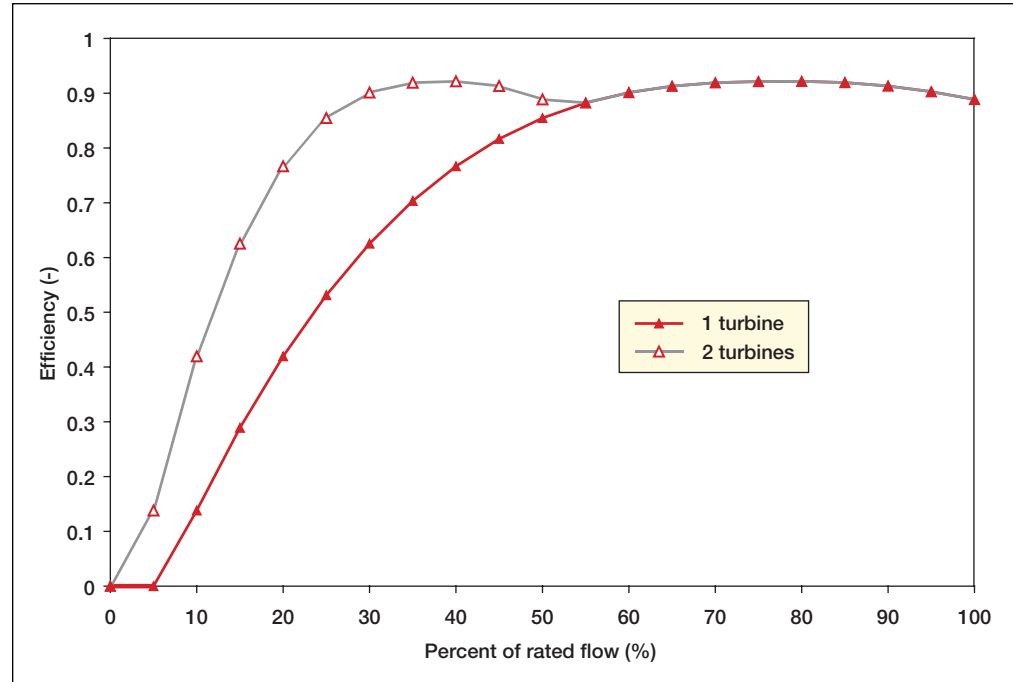


Figure 10:
 Calculated Efficiency Curves for Francis Turbine
 (Gross Head = 146 m; Design Flow = 1.90 m³/s).

2.3.2 Power available as a function of flow

Actual power P available from the small hydro plant at any given flow value Q is given by the following equation, in which the flow-dependent hydraulic losses and tailrace reduction are taken into account:

$$P = \rho g Q \left[H_g - (h_{hydr} + h_{tail}) \right] e_t e_g (1 - l_{trans})(1 - l_{para}) \quad (7)$$

where ρ is the density of water (1,000 kg/m³), g the acceleration of gravity (9.81 m/s²), H_g the gross head, h_{hydr} and h_{tail} are respectively the hydraulic losses and tailrace effect associated with the flow; and e_t is the turbine efficiency at flow Q , calculated as explained in Section 2.3.1. Finally, e_g is the generator efficiency, l_{trans} the transformer losses, and l_{para}



the parasitic electricity losses; e_g , l_{trans} , and l_{para} are specified by the user in the *Energy Model* worksheet and are assumed independent from the flow considered.

Hydraulic losses are adjusted over the range of available flows based on the following relationship:

$$h_{hydr} = H_g l_{hydr,max} \frac{Q^2}{Q_{des}^2} \quad (8)$$

where $l_{hydr,max}$ is the maximum hydraulic losses specified by the user, and Q_{des} the design flow. Similarly the maximum tailrace effect is adjusted over the range of available flows with the following relationship:

$$h_{tail} = h_{tail,max} \frac{(Q - Q_{des})^2}{(Q_{max} - Q_{des})^2} \quad (9)$$

where $h_{tail,max}$ is the maximum tailwater effect, i.e. the maximum reduction in available gross head that will occur during times of high flows in the river. Q_{max} is the maximum river flow, and equation (9) is applied only to river flows that are greater than the plant design flow (i.e. when $Q > Q_{des}$).

2.3.3 Plant capacity

Plant capacity P_{des} is calculated by re-writing equation (7) at the design flow Q_{des} . The equation simplifies to:

$$P_{des} = \rho g Q_{des} H_g (1 - l_{hydr}) e_{t,des} e_g (1 - l_{trans}) (1 - l_{para}) \quad (10)$$

where P_{des} is the plant capacity and $e_{t,des}$ the turbine efficiency at design flow, calculated as explained in Section 2.3.1.

The small hydro plant firm capacity is calculated again with equation (7), but this time using the firm flow and corresponding turbine efficiency and hydraulic losses at this flow. If the firm flow is greater than the design flow, firm plant capacity is set to the plant capacity calculated through equation (10).



2.3.4 Power-duration curve

Calculation of power available as a function of flow using equation (7) for all 21 values of the available flow $Q'_0, Q'_5, \dots, Q'_{100}$ used to define the flow-duration curve, leads to 21 values of available power P_0, P_1, \dots, P_{100} defining a power-duration curve. Since the design flow is defined as the maximum flow that can be used by the turbine, the flow values used in equations (7) and (8) are actually $Q_{n,used}$ defined as⁴:

$$Q_{n,used} = \min(Q'_n, Q_{des}) \tag{11}$$

An example power-duration curve is shown in **Figure 11**, with the design flow equal to 3 m³/s.

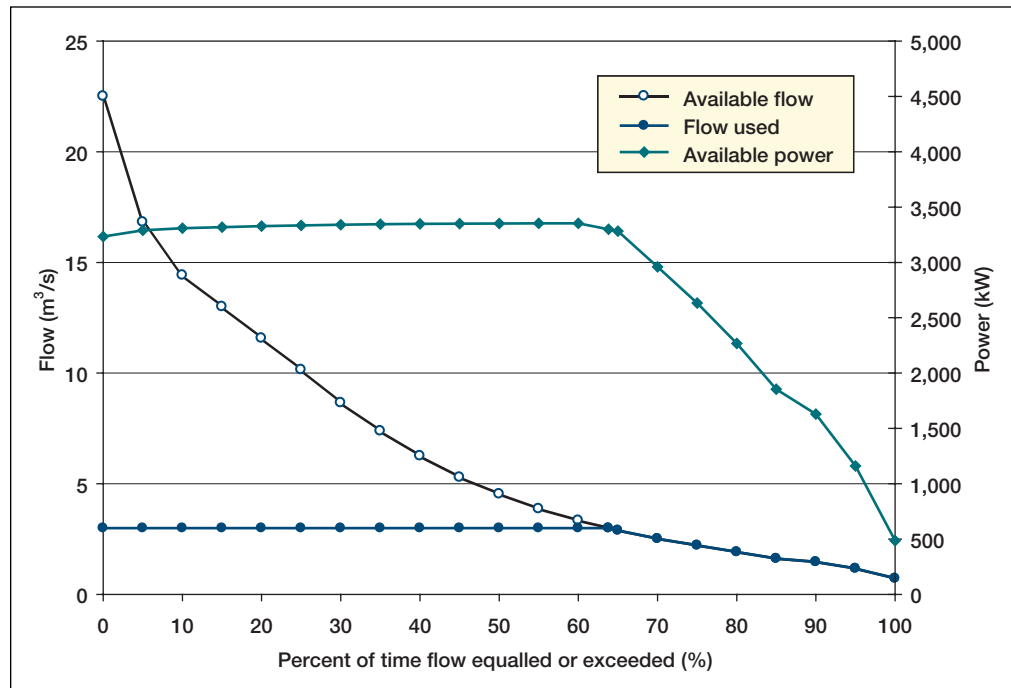


Figure 11:
Example of a Power-Duration Curve.

4. In equation (9), however, neither the residual flow nor the maximum flow should be taken into account, and it is indeed Q_n that should be used, not $Q_{n,used}$.



2.3.5 Renewable energy available

Renewable energy available is determined by calculating the area under the power curve assuming a straight-line between adjacent calculated power output values. Given that the flow-duration curve represents an annual cycle, each 5% interval on the curve is equivalent to 5% of 8,760 hours (number of hours per year). The annual available energy E_{avail} (in kWh/yr) is therefore calculated from the values P (in kW) by:

$$E_{avail} = \sum_{k=1}^{20} \left(\frac{P_{5(k-1)} + P_{5k}}{2} \right) \frac{5}{100} 8760 (1 - l_{dt}) \quad (12)$$

where l_{dt} is the annual downtime losses as specified by the user. In the case where the design flow falls between two 5% increments on the flow-duration curve (as in **Figure 11**) the interval is split in two and a linear interpolation is used on each side of the design flow.

Equation (12) defines the amount of renewable energy available. The amount actually delivered depends on the type of grid, as is described in the following sections.

2.3.6 Renewable energy delivered - central-grid

For central-grid applications, it is assumed that the grid is able to absorb all the energy produced by the small hydro power plant. Therefore, all the renewable energy available will be delivered to the central-grid and the renewable energy delivered, E_{dvd} , is simply:

$$E_{dvd} = E_{avail} \quad (13)$$

2.3.7 Renewable energy delivered - isolated-grid and off-grid

For isolated-grid and off-grid applications the procedure is slightly more complicated because the energy delivered is actually limited by the needs of the local grid or the load, as specified by the load-duration curve (**Figure 9**). The following procedure is used: for each 5% increment on the flow-duration curve, the corresponding available plant power output (assumed to be constant over a day) is compared to the load-duration curve (assumed to represent the daily load demand). The portion of energy that can be delivered by the small hydro plant is determined as the area that is under both the load-duration curve and the horizontal line representing the available plant power output. Twenty-one values of the daily energy delivered G_0, G_5, \dots, G_{100} corresponding to available power P_0, P_5, \dots, P_{100} are calculated. For each value of available power P_n , daily energy delivered G_n is given by:



$$G_n = \sum_{k=1}^{20} \left(\frac{P'_{n,5(k-1)} + P'_{n,5k}}{2} \right) \frac{5}{100} 24 \quad (14)$$

where $P'_{n,k}$ is the lesser of load L_k and available power P_n :

$$P'_{n,k} = \min(P_n, L_k) \quad (15)$$

In the case where the available power $P'_{n,k}$ falls between two 5% increments on the load-duration curve, the interval is split in two and a linear interpolation is used on each side of the available power.

The procedure is illustrated by an example, using the load-duration curve from **Figure 9** and values from the power-duration curve shown in **Figure 11**. The purpose of the example is to determine the daily renewable energy G_{75} delivered for a flow that is exceeded 75% of the time. One first refers to **Figure 11** to determine the corresponding power level:

$$P_{75} = 2,630 \text{ kW} \quad (16)$$

Then one reports that number as a horizontal line on the load-duration curve, as shown in **Figure 12**. The area that is both under the load-duration curve and the horizontal line is the renewable energy delivered per day for the plant capacity that corresponds to flow Q_{75} ; integration with formula (14) gives the result:

$$G_{75} = 56.6 \text{ MWh/d} \quad (17)$$

The procedure is repeated for all values P_0, P_5, \dots, P_{100} to obtain twenty one values of the daily renewable energy delivered G_0, G_5, \dots, G_{100} as a function of percent time the flow is exceeded as shown in **Figure 13**. The annual renewable energy delivered E_{dlyd} is obtained simply by calculating the area under the curve of **Figure 13**, again with a trapezoidal rule:

$$E_{dlyd} = \sum_{n=1}^{20} \left(\frac{G_{5(n-1)} + G_{5n}}{2} \right) \frac{5}{100} 365 (1 - l_{dt}) \quad (18)$$

where, as before, l_{dt} is the annual downtime losses as specified by the user.

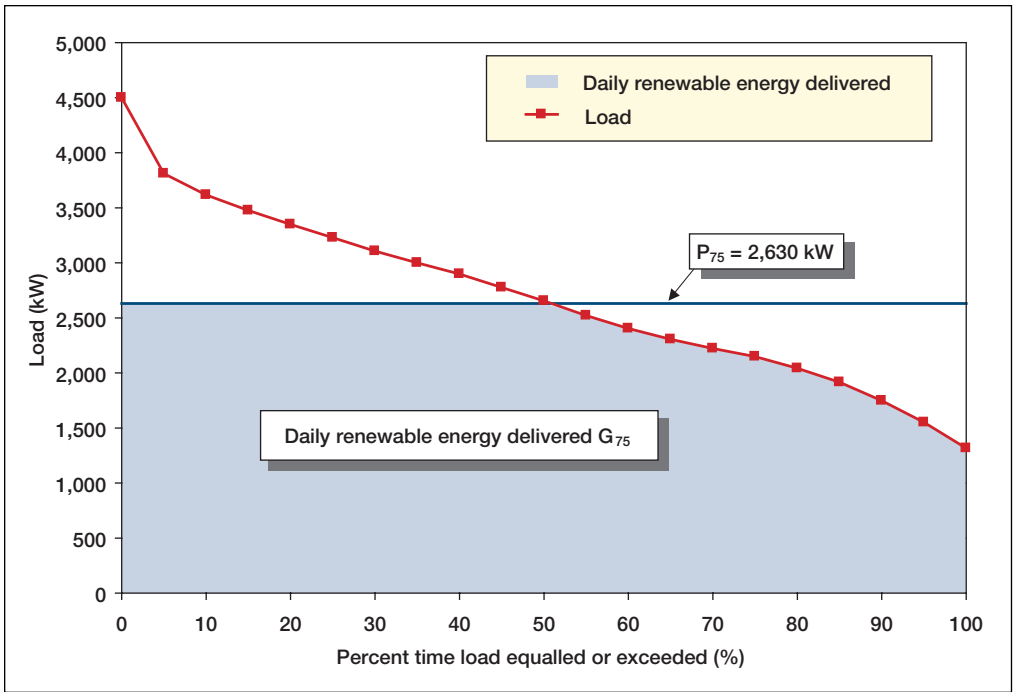


Figure 12:
Example of Calculation of Daily Renewable Energy Delivered.

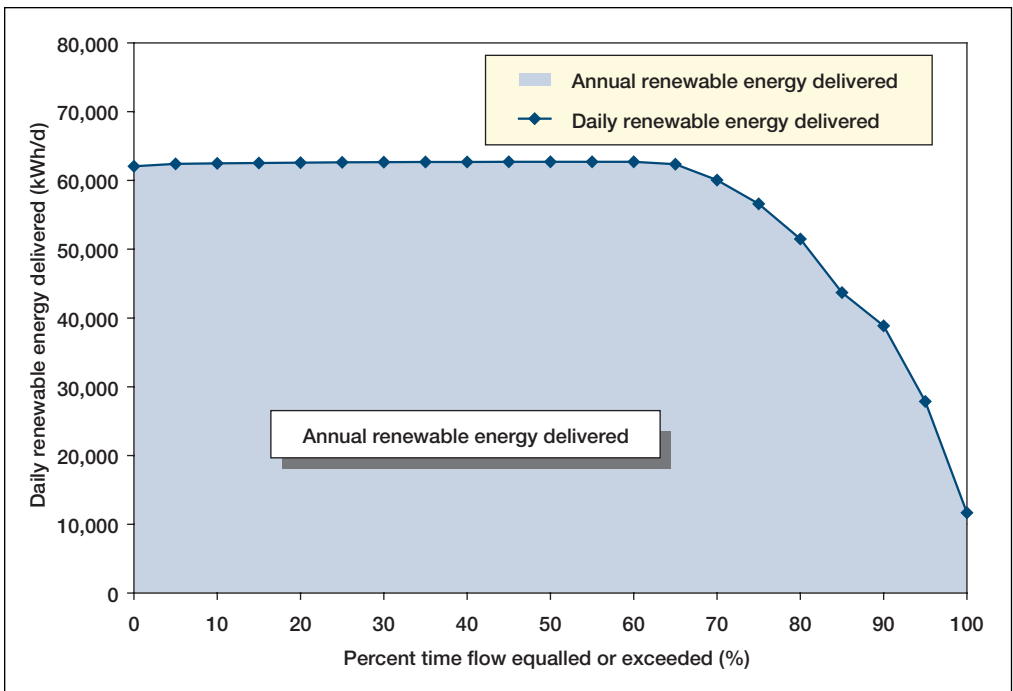


Figure 13:
Example of Calculation of Annual Renewable Energy Delivered.



2.3.8 Small hydro plant capacity factor

The annual capacity factor K of the small hydro power plant is a measure of the available flow at the site and how efficiently it is used. It is defined as the average output of the plant compared to its rated capacity:

$$K = \frac{E_{dvd}}{8760 P_{des}} \quad (19)$$

where the annual renewable energy delivered E_{dvd} calculated through (13) or (18) is expressed in kWh, and plant capacity calculated through (10) is expressed in kW.

2.3.9 Excess renewable energy available

Excess renewable energy available E_{excess} is the difference between the renewable energy available E_{avail} and the renewable energy delivered E_{dvd} :

$$E_{excess} = E_{avail} - E_{dvd} \quad (20)$$

E_{avail} is calculated through equation (12) and E_{dvd} through either (13) or (18).

2.4 Project Costing

The Small Hydro Project Model is unique among RETScreen technology models in that it offers two methods for project costing: the detailed costing method, or alternatively, the formula costing method.

The detailed costing method is described in the online user manual. The formula costing method is based on empirical formulae that have been developed to relate project costs to key project parameters. The costs of numerous projects have been used to develop the formulae. The formulae are described in *Appendix B*.



2.5 Validation

Numerous experts have contributed to the development, testing and validation of the RETScreen Small Hydro Project Model. They include small hydro modelling experts, cost engineering experts, greenhouse gas modelling specialists, financial analysis professionals, and ground station (hydrology) and satellite weather database scientists.

This section presents three examples of the validations completed. In *Section 2.5.1*, a turbine efficiency curve as calculated by RETScreen is compared to manufacturer's efficiency data for an installed unit with the same characteristics. Then, the annual renewable energy delivered and plant capacity calculated by RETScreen are compared to values calculated by another software program in *Section 2.5.2*. And finally, project costs, as calculated by the formula costing method, are compared to the as-built costs of one small hydro project in *Section 2.5.3*.

2.5.1 Turbine efficiency

Small hydro turbine efficiency as calculated by RETScreen was compared to the manufacturer guaranteed turbine efficiency for the Brown Lake Hydro Project in British Columbia, Canada.

The following provides a summary of the Brown Lake project and the turbine performance data as provided by the manufacturer:

- **Project name:**
Brown Lake Hydro Project
- **Project location:**
Approximately 40 km south of Prince Rupert, British Columbia on the confluence of Brown Creek and Ecstall River.
- **Project features:**
600 m rock tunnel tapping into Brown Lake, 50 m of 1.5 m diameter steel penstock, single horizontal Francis turbine, horizontal synchronous generator, 1,500 m of submarine power cable, substation and connection to BC Hydro at 69 kV. Automatic operation and remote monitoring.
- **Date commissioned:**
December 1996
- **Turbine manufacturer:**
GEC Alsthom (runner by Neyrpic)
- **Turbine type:**
Francis



- **Nameplate rating:**
6,870 kW at 103.6 m net head
- **Maximum rated power:**
7,115 kW at 105.6 m net head
- **RPM:**
514
- **Diameter:**
1,100 mm
- **Number of blades:**
13
- **Efficiency data:**
(see *Table 1*)

Flow (m ³ /s)	Efficiency
7.35	0.93
7.00	0.93
6.65	0.93
6.30	0.92
5.95	0.91
5.60	0.90
5.25	0.90
4.90	0.88
4.55	0.87
4.20	0.85
3.85	0.84
3.50	0.82

Table 1: Manufacturer's Turbine Efficiency Data.

A gross head value of 109.1 m was entered into RETScreen, which corresponds to a net head of 103.6 m with maximum hydraulic losses of 5%. Comparison between the manufacturer's efficiency data and the efficiency curve generated by RETScreen is shown in *Figure 14*. As illustrated in the figure, the RETScreen calculated efficiency curve provides a good approximation of the as-designed turbine efficiencies.

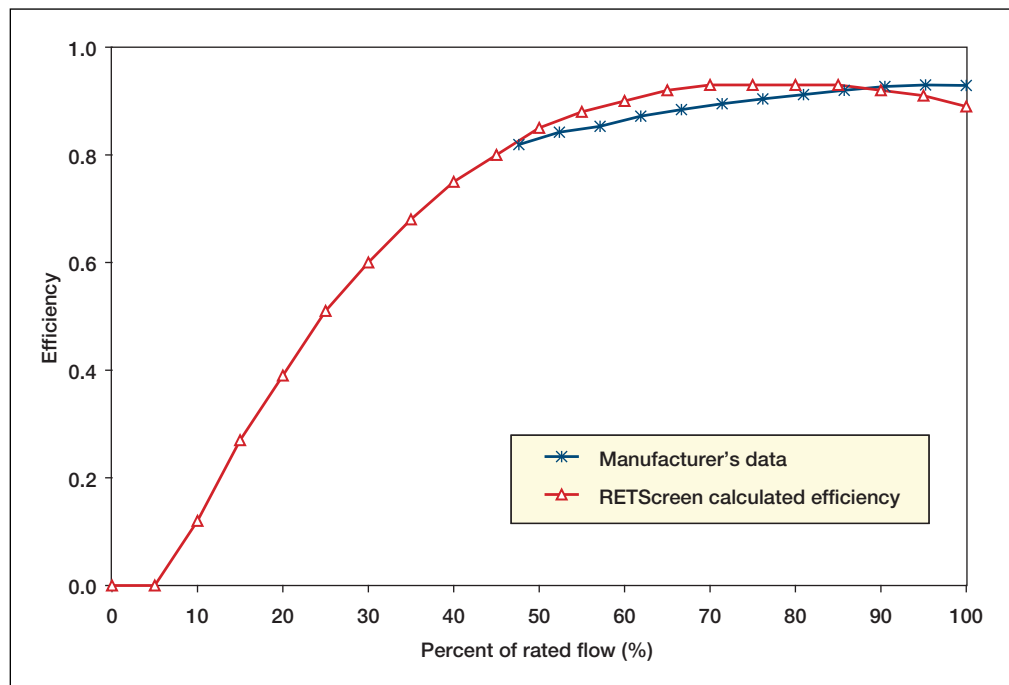


Figure 14:
Comparison of RETScreen Calculated Hydro Turbine Efficiency against Manufacturer's Data.



Note that detailed on-site index testing would be required to verify the manufacturers as-designed efficiency curve. Accurate index tests are very costly and not normally undertaken for small hydro projects unless there is sufficient concern that the turbine is not performing as designed. An index test would likely yield some differences in the shape of the manufacturer's efficiency curve.

2.5.2 Plant capacity and annual renewable energy delivered

A comparison between the RETScreen Small Hydro Project Model and another software program called HydrA is presented in a report for the International Energy Agency – Implementing Agreement for Hydropower Technologies and Programmes entitled “Assessment Methods for Small-hydro Projects” by E.M. Wilson, D.Sc., FICE, FASCE, dated April 2000. HydrA is a software package used to estimate the hydropower potential at any location in the United Kingdom or Spain. HydrA incorporates a regional flow estimation model derived from extensive statistical analysis of national river flow data and catchment information.

The following is extracted from the report:

Comparison of the RETScreen⁵ and HydrA energy analyses was made for a Scottish catchment where the HydrA-derived flow-duration curve was entered in RETScreen. The standard generic efficiency curves in both programs were left unchanged, although these differ to some extent. Rated flow and residual flows [sic] were made the same. The resulting annual energy values were obtained:

Mean flow:	1.90 m³/s
Residual flow:	0.27 m³/s
Rated turbine flow:	1.63 m³/s
Gross hydraulic head:	65.0 m
Net hydraulic head:	58.5 m

Applicable Turbines	Gross Annual Av. Output MWh	Net Annual Av. Output MWh	Maximum Power Output kW	Rated Capacity kW	Minimum Operational Flow m ³ /s
RETScreen					
Francis		3 092		819.0	
Crossflow		2 936		745.0	
Turgo		3 125		758.0	
HydrA					
Francis	3 270.3	3 107	858.7	824.4	0.76
Crossflow	3 072.7	2 919	748.3	700.5	0.51
Turgo	3 163.1	3 005	809.1	728.2	0.43

It may be concluded from this simple test that there is little difference in the energy calculations.

5. A beta version of the RETScreen Small Hydro Project Model Version 2000 was used for the test.

2.5.3 Project costs

Project costs as calculated by RETScreen using the Formula Costing Method were compared to a detailed as-built cost evaluation prepared for the existing 6 MW Rose Blanche hydroelectric development in Newfoundland, Canada.

The key parameters of the Rose Blanche project are summarised below:

- **Project name:**
Rose Blanche Hydroelectric Development
- **Owner/developer:**
Newfoundland Power
- **Project location:**
Rose Blanche Brook, approximately 45 km east of Channel Port Aux Basques.
- **Date commissioned:**
December 1998
- **Project type:**
Run-of-river (with several days' storage)
- **Installed capacity:**
6 MW
- **Design net head:**
114.2 m
- **Rated flow:**
6.1 m³/s
- **Turbine/generator:**
Twin Francis turbines connected to a single generator.
- **Other project features:**
Small dam with minimal storage, 1,300 m penstock, short transmission line (approximately 3 km).

The data inputs for the RETScreen Formula Costing Method and the results are shown in *Figure 15*, and a comparison of the costs as calculated by RETScreen and the detailed cost evaluation for the real project is presented in *Figure 16*. The detailed project costs estimated in 1998 have been converted to 2000 values using an inflation factor of 1.03.

RETScreen[®] Cost Analysis - Small Hydro Project [Search Marketplaces](#)

Costing method: Currency: Cost references:

Formula Costing Method			Notes/Range	
Input Parameters				
Project country		Canada		
Cold climate?	yes/no	Yes		
Frost days at site	day	200	See Map	Visit NASA satellite data site
Number of turbines	turbine	2		
Flow per turbine	m ³ /s	3.1		
Approx. turbine runner diameter (per unit)	m	0.8		
Project classification:				
Suggested classification	-	Mini		
Selected classification	-	Small		
Existing dam?	yes/no	No		
New dam crest length	m	250.0		
Rock at dam site?	yes/no	Yes		
Maximum hydraulic losses	%	5%		
Intake and miscellaneous losses	%	1%		1% to 5%
Access road required?	yes/no	Yes		
Length	km	5.0		
Tote road only?	yes/no	No		
Difficulty of terrain	-	3.0		1.0 to 6.0
Tunnel required?	yes/no	No		
Canal required?	yes/no	No		
Penstock required?	yes/no	Yes		
Length	m	1,300.0		
Number of identical penstocks	penstock	1		
Allowable penstock headloss factor	%	4.0%		1.0% to 4.0%
Pipe diameter	m	1.61		
Average pipe wall thickness	mm	8.1		
Distance to borrow pits	km	3.0		
Transmission line				
Length	km	5.0		
Difficulty of terrain	-	1.0		1.0 to 2.0
Voltage	kV	44.0		
Interest rate	%	9.0%		

Initial Costs (Formula Method)	Cost (local currency)	Adjustment Factor	Amount (local currency)	Relative Costs
Feasibility Study	\$ 504,000	1.00	\$ 504,000	3.1%
Development	\$ 529,000	1.00	\$ 529,000	3.3%
Land rights			\$ -	0.0%
Development Sub-total:			\$ 529,000	3.3%
Engineering	\$ 537,000	1.00	\$ 537,000	3.3%
Energy Equipment	\$ 3,032,000	1.00	\$ 3,032,000	18.6%
Balance of Plant			\$ 1,096,000	6.7%
Access road	\$ 1,096,000	1.00	\$ 1,096,000	6.7%
Transmission line	\$ 217,000	1.00	\$ 217,000	1.3%
Substation and transformer	\$ 175,000	1.00	\$ 175,000	1.1%
Penstock	\$ 1,831,000	1.00	\$ 1,831,000	11.3%
Canal	\$ -	1.00	\$ -	0.0%
Tunnel	\$ -	1.00	\$ -	0.0%
Civil works (other)	\$ 6,326,000	1.00	\$ 6,326,000	38.9%
Balance of Plant Sub-total:	\$ 9,645,000		\$ 9,645,000	59.3%
Miscellaneous	\$ 2,015,000	1.00	\$ 2,015,000	12.4%
GHG baseline study and MP	Cost \$ -		\$ -	0.0%
GHG validation and registration	Cost \$ -		\$ -	0.0%
Miscellaneous Sub-total:			\$ 2,015,000	12.4%
Initial Costs - Total (Formula Method)	\$ 16,262,000		\$ 16,262,000	100.0%

Figure 15:
Cost Analysis Worksheet for Rose Blanche Hydroelectric Project.

The RETScreen Formula Costing Method calculated total cost is approximately 14% higher than the detailed project cost evaluation for the real project. The RETScreen estimate, however, includes a cost for the feasibility study, which is not part of the detailed cost estimate. If the feasibility cost is deducted from the RETScreen estimate, the difference in results reduces to 11% (RETScreen results being 11% higher than the detailed cost estimate).

For the RETScreen Formula Costing Method the project classification was selected as “small” to represent the higher design and construction standards that would normally be attributable to projects designed and constructed by a large utility. If the recommended project classification of “mini” were used, and the feasibility study cost removed, the RETScreen estimate would be approximately 9% lower than the detailed cost evaluation.

Rose Blanche Hydroelectric Development, Newfoundland, Canada						
RETScreenfi Formula Costing Method				(1)	Detailed Project Costs	Variance
Initial Costs (Formula Method)	Cost (local currency)	Adjustment Factor	Amount (local currency)	(2)		(1)/(2)
				Amount (local currency)		
Feasibility Study	\$ 504,000	1.00	\$ 504,000	\$ -		
Development	\$ 529,000	1.00	\$ 529,000	\$ 463,500		114%
Land rights			\$ -	\$ -		
Development Sub-total:			\$ 529,000	\$ 463,500		114%
Engineering	\$ 537,000	1.00	\$ 537,000	\$ 875,500		61%
Energy Equipment	\$ 3,032,000	1.00	\$ 3,032,000	\$ 2,729,500		111%
Balance of Plant						
Access road	\$ 1,096,000	1.00	\$ 1,096,000	\$ 957,900		114%
Transmission line	\$ 217,000	1.00	\$ 217,000	\$ 372,860		58%
Substation and transformer	\$ 175,000	1.00	\$ 175,000	\$ 539,720		32%
Penstock	\$ 1,831,000	1.00	\$ 1,831,000	\$ 3,090,000		59%
Canal	\$ -	1.00	\$ -	\$ -		
Tunnel	\$ -	1.00	\$ -	\$ -		
Civil works (other)	\$ 6,326,000	1.00	\$ 6,326,000	\$ 4,351,750		145%
Balance of Plant Sub-total:	\$ 9,645,000		\$ 9,645,000	\$ 9,312,230		104%
Miscellaneous	\$ 2,015,000	1.00	\$ 2,015,000	\$ 821,940		245%
GHG baseline study and MP	Cost \$ -		\$ -	\$ -		
GHG validation and registration	Cost \$ -		\$ -	\$ -		
Miscellaneous Sub-total:			\$ 2,015,000	\$ 821,940		245%
Initial Costs - Total (Formula Method)	\$ 16,262,000		\$ 16,262,000	\$ 14,202,670		114%

Figure 16:
Comparison of Costs Calculated Using RETScreen Formula Method vs. Detailed Project Costs.

While there are some discrepancies in the details between the two cost estimates, overall the totals correspond well. Some of the discrepancies could be explained by a different cost categorisation that was used for the detailed evaluation (grouping of certain categories of the detailed estimate were required in order to match the RETScreen categories). The accuracy of the cost estimate by the RETScreen Small Hydro Project Model Formula Costing Method is nevertheless sufficient at the pre-feasibility stage of a study.

2.6 Summary

In this section the algorithms used by the RETScreen Small Hydro Project Model have been shown in detail. Generic formulae enable the calculation of turbine efficiency for a variety of turbines. These efficiencies, together with the flow-duration curve and (in the case of isolated-grid and off-grid applications) the load-duration curve, enable the calculation of renewable energy delivered by a proposed small hydro power plant. Condensed formulae enable the estimation of project costs; alternatively, a detailed costing method can be used. The accuracy of the model, with respect to both energy production and cost estimation, is excellent for pre-feasibility stage studies for small hydro projects.



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APPENDIX A – TURBINE EFFICIENCY FORMULAE

FRANCIS, KAPLAN AND PROPELLOR TURBINES (REACTION TURBINES):	
ITEM	FORMULA
Reaction turbine runner size (d)	$d = kQ_d^{0.473}$ <p>where: d = runner throat diameter in m k = 0.46 for $d < 1.8$ = 0.41 for $d \geq 1.8$ Q_d = design flow (flow at rated head and full gate opening in m³/s)</p>
Specific speed (n_q)	$n_q = kh^{-0.5}$ <p>where: n_q = specific speed based on flow k = 800 for propeller and Kaplan turbines = 600 for Francis turbines h = rated head on turbine in m (gross head less maximum hydraulic losses)</p>



FRANCIS TURBINES:	
ITEM	FORMULA
Specific speed adjustment to peak efficiency (\hat{e}_{nq})	$\hat{e}_{nq} = \left\{ (n_q - 56) / 256 \right\}^2$
Runner size adjustment to peak efficiency (\hat{e}_d)	$\hat{e}_d = (0.081 + \hat{e}_{nq}) (1 - 0.789d^{-0.2})$
Turbine peak efficiency (e_p)	$e_p = (0.919 - \hat{e}_{nq} + \hat{e}_d) - 0.0305 + 0.005 R_m$ <p>where: R_m = turbine manufacture/design coefficient (2.8 to 6.1; default = 4.5). Refer to online manual.</p>
Peak efficiency flow (Q_p)	$Q_p = 0.65 Q_d n_q^{0.05}$
Efficiencies at flows below peak efficiency flow (e_q)	$e_q = \left\{ 1 - \left[1.25 \left(\frac{(Q_p - Q)}{Q_p} \right)^{(3.94 - 0.0195n_q)} \right] \right\} e_p$
Drop in efficiency at full load (\hat{e}_p)	$\hat{e}_p = 0.0072 n_q^{0.4}$
Efficiency at full load (e_r)	$e_r = (1 - \hat{e}_p) e_p$
Efficiencies at flows above peak efficiency flow (e_q)	$e_q = e_p - \left[\left(\frac{Q - Q_p}{Q_d - Q_p} \right)^2 (e_p - e_r) \right]$



KAPLAN AND PROPELLOR TURBINES:	
ITEM	FORMULA
Specific speed adjustment to peak efficiency (\hat{e}_{nq})	$\hat{e}_{nq} = \left\{ (n_q - 170) / 700 \right\}^2$
Runner size adjustment to peak efficiency (\hat{e}_d)	$\hat{e}_d = (0.095 + \hat{e}_{nq}) (1 - 0.789d^{-0.2})$
Turbine peak efficiency (e_p)	$e_p = (0.905 - \hat{e}_{nq} + \hat{e}_d) - 0.0305 + 0.005 R_m$ <p>where: R_m = Turbine manufacture/design coefficient (2.8 to 6.1; default 4.5). Refer to online manual.</p>

KAPLAN TURBINES:	
ITEM	FORMULA
Peak efficiency flow (Q_p)	$Q_p = 0.75 Q_d$
Efficiency at flows above and below peak efficiency flow (e_q)	$e_q = \left[1 - 3.5 \left(\frac{Q_p - Q}{Q_p} \right)^6 \right] e_p$

PROPELLOR TURBINES:	
ITEM	FORMULA
Peak efficiency flow (Q_p)	$Q_p = Q_d$
Efficiencies at flows below peak efficiency flow (e_q)	$e_q = \left[1 - 1.25 \left(\frac{Q_p - Q}{Q_p} \right)^{1.13} \right] e_p$



PELTON TURBINES:	
ITEM	FORMULA
Rotational speed (n)	$n = 31 \left(h \frac{Q_d}{j} \right)^{0.5}$ <p>where: j = Number of jets (user-selected value from 1 to 6)</p>
Outside diameter of runner (d)	$d = \frac{49.4 h^{0.5} j^{0.02}}{n}$
Turbine peak efficiency (e_p)	$e_p = 0.864 d^{0.04}$
Peak efficiency flow (Q_p)	$Q_p = (0.662 + 0.001j) Q_d$
Efficiency at flows above and below peak efficiency flow (e_q)	$e_q = \left[1 - \left\{ (1.31 + 0.025j) \left \left(\frac{Q_p - Q}{Q_p} \right) \right ^{(5.6 + 0.4j)} \right\} \right] e_p$

TURGO TURBINES:	
ITEM	FORMULA
Efficiency (e_q)	Pelton efficiency minus 0.03

CROSS-FLOW TURBINES:	
ITEM	FORMULA
Peak efficiency flow (Q_p)	$Q_p = Q_d$
Efficiency (e_q)	$e_q = 0.79 - 0.15 \left(\frac{Q_d - Q}{Q_p} \right) - 1.37 \left(\frac{Q_d - Q}{Q_p} \right)^{14}$

APPENDIX B – FORMULAE FOR FORMULA COSTING METHOD

VARIABLES LISTED ALPHABETICALLY					
A	Access road difficulty factor	J_t	Higher cost vertical axis turbine factor	n_p	Number of penstocks
B	Foreign costs civil works factor	k	Allowable tunnel headloss (ratio to H _g)	P	Transmission line wood pole vs. steel tower factor
C	Civil cost factor	K	User-defined equipment manufacture cost coefficient to account for country of manufacture	Q	Flow under consideration (m ³ /s)
C_g	Lower cost generator factor	K_t	Lower cost small horizontal axis turbine factor	Q_d	Design flow (m ³ /s)
C_v	Tunnel volume of concrete lining (m ³)	I_a	Access road length (km)	Q_u	Flow per unit (m ³ /s)
d	Runner diameter (m)	I_b	Distance to borrow pits (km)	R	Rock factor
D	Transmission line difficulty factor	L_c	Ratio of the cost of local labour costs compared to Canadian cost expressed as a decimal	R_v	Tunnel volume of rock excavation (m ³)
d_p	Diameter of penstock(s) (m)	I_{cr}	Canal length in rock (m)	S_r	Side slope of rock terrain through which canal will be built (degrees)
E	Engineering cost factor	I_{cs}	Canal length in impervious soil (m)	S_s	Side slope of soil terrain through which canal will be built (degrees)
E_c	Ratio of the cost of local construction equipment costs compared to Canadian costs expressed as a decimal	I_d	Dam crest length (m)	T	Tote road factor
f	Frost days at site	I_p	Penstock length (m)	t_{ave}	Average penstock thickness (mm)
F	Frost days factor	I_T	Transmission line length (km)	t_b	Penstock thickness at turbine (mm)
F_c	Ratio of the cost of local fuel costs compared to Canadian costs expressed as a decimal	I_t	Tunnel length (m)	T_c	Tunnel lining length ratio
G	Grid connected factor	MW	Total capacity (MW)	t_t	Penstock thickness at intake (mm)
H_g	Gross head (m)	MW_u	Capacity per unit (MW)	V	Transmission line voltage (kV)
i	Interest rate (%)	n	Number of turbines	W	Penstock weight (steel) (kg)

BASIC PARAMETERS			
ITEM	SMALL	MINI	MICRO
Design flow (maximum flow used by generating station) in m ³ /s (Q_d)	User-defined value		
Recommended classification	$Q_d > 12.8$	$12.8 \geq Q_d > 0.4$	$Q_d \leq 0.4$
Selected classification	User-defined value based on acceptable risk (flood, etc.).		
Number of turbines (n)	User-defined value		
Flow per turbine in m ³ /s (Q_u)	$= Q_d / n$		
Approx. turbine runner diameter in m (d)	$= 0.482 Q_u^{0.45}$		
Gross head in m (H_g)	User-defined value		
MW/unit in MW (hidden) (MW_u)	$= 8.22 Q_u H_g / 1000$	$= 7.79 Q_u H_g / 1000$	$= 7.53 Q_u H_g / 1000$
Total Capacity in MW (hidden) (MW)	$= MW_u n$		



OTHER VARIABLES AND COSTING FACTORS (IN ORDER OF USE IN FORMULAE)			
ITEM	SMALL	MINI	MICRO
Engineering cost factor (hidden) (E)		= 0.67 if existing dam = 1.0 if no dam as specified by yes/no selection	
Grid-connected factor to account for the use of induction generators (hidden) (G)		= 0.9 if MW < 1.5 and central-grid connected	
Factor to account for use of lower-cost motors as generators for projects below 10 MW (hidden) (C_g)		= 0.75 if MW < 10 = 1.0 if MW ≥ 10	
Factor to account for cost increase with vertical axis Kaplan, Francis and Propeller units at heads above 25 m (hidden) (J₁)		= 1 if H _g ≤ 25 = 1.1 if H _g > 25	
Factor to account for cost decrease with small horizontal Kaplan, Francis and Propeller axis units (hidden) (K₁)		= 0.9 if d < 1.8 = 1.0 if d ≥ 1.8	
Factor to adjust access road costs to reflect lower-cost tote road construction (hidden) (T)		= 0.25 if tote road = 1.0 otherwise as specified by yes/no selection	
Access road difficulty of terrain factor (A)		User-defined factor with recommended range of 1 to 6	
Length of access road in m (l_a)		User-defined value	





OTHER VARIABLES AND COSTING FACTORS (IN ORDER OF USE IN FORMULAE)			
ITEM	SMALL	MINI	MICRO
Transmission line difficulty of terrain factor (D)	User-defined factor with recommended range of 1 to 2		
Length of transmission line in km (l _t)	User-defined value		
Transmission line voltage in kV (V)	User-defined value		
Factor to reflect cost of wood pole vs. steel tower construction (hidden) (P)	= 0.85 if V < 69 = 1.0 if V ≥ 69		
Civil cost factor (hidden) (C)	= 0.44 if existing dam = 1.0 if no dam as specified by yes/no selection		
Rock factor (hidden) (R)	= 1 if rock at dam site = 1.05 if no rock as specified by yes/no selection		N/A
Distance to a borrow pit in km (l _b)	User-defined value		
Length of dam crest in m (l _d)	User-defined value		
Number of identical penstocks (n _p)	User-defined value		
Weight of penstock(s) in kg (hidden) (W)	Calculated value (penstock cost formula)		
Diameter of penstock(s) in m (d _p)	Calculated value (penstock cost formula)		
Length of penstock(s) in m (l _p)	User-defined value		





OTHER VARIABLES AND COSTING FACTORS (IN ORDER OF USE IN FORMULAE)			
ITEM	SMALL	MINI	MICRO
Average pipe wall thickness of penstock(s) in mm (t_{ave})		Calculated value (penstock cost formula)	
Penstock pipe wall thickness at intake in mm (hidden) (t_i)		Calculated value (penstock cost formula)	
Penstock pipe wall thickness at turbine in mm (hidden) (t_b)		Calculated value (penstock cost formula)	
Terrain side slope of soil through which canal is to be constructed in degrees (S_s)		User-defined value	
Length of canal to be constructed in soil in m (l_{cs})		User-defined value	
Terrain side slope of rock through which canal is to be constructed in degrees (S_r)		User-defined value	
Length of canal to be constructed in rock in m (l_{cr})		User-defined value	
Tunnel rock excavation volume in m ³ (hidden) (R_v)		Calculated value (tunnel cost formula)	N/A
Tunnel concrete lining volume in m ³ (hidden) (C_v)		Calculated value (tunnel cost formula)	N/A
Length of tunnel in m (l_t)		User-defined value	N/A





OTHER VARIABLES AND COSTING FACTORS (IN ORDER OF USE IN FORMULAE)			
ITEM	SMALL	MINI	MICRO
Allowable tunnel headloss expressed as a ratio to the gross head (k)	User-defined value		N/A
Percent length of tunnel that is lined factor (T _c)	User-defined value with recommended range of 15% (excellent rock) to 100% (poor rock)		N/A
Interest rate (i)	User-defined value		
Number of days with frost at site (f)	User-defined value		
Frost-days factor (hidden) (F)	$= \frac{110}{(365 - f)^{0.9}}$		
Local vs. Canadian equipment costs ratio (E _c)	User-defined value		
Local vs. Canadian fuel costs ratio (F _c)	User-defined value		
Local vs. Canadian labour costs ratio (L _c)	User-defined value		
Civil works foreign cost factor (hidden). Used in program to determine local cost of the civil works components of foreign projects (B)	$= (0.3333E_c + 0.3333F_c) \times \frac{1}{\left(\frac{E_c}{L_c}\right)^{0.5} + 0.3333} \left(\frac{E_c}{L_c}\right)^{0.5} L_c$		
Equipment manufacture cost coefficient (K)	User-defined value with recommended range of 0.5 to 1.0		



BASIC COSTING FORMULAE				
ITEM	SMALL	MINI	MICRO	
Feasibility study (Eq.#1)	$= 0.032 \sum (Eq.\#2) \text{ to } (Eq.\#15)$			
Development (Eq.#2)	$= 0.04 \sum (Eq.\#3) \text{ to } (Eq.\#14)$			
Engineering (Eq.#3)	$= 0.37 n^{0.1} E \left(\frac{MW}{H_g^{0.3}} \right)^{0.54} \times 10^6$		$= 0.04 \left(\frac{MW}{H_g^{0.3}} \right)^{0.54} \times 10^6$	
Energy equipment (Eq.#4)	Generator and Control: (all turbine types)	$= 0.82 n^{0.96} G C_g \left(\frac{MW}{H_g^{0.28}} \right)^{0.9} \times 10^6$		
	Kaplan turbine and governor:	$= 0.27 n^{0.96} J_i K_i d_i^{1.47} (1.17 H_g^{0.12} + 2) \times 10^6$		
	Francis turbine and governor:	$= 0.17 n^{0.96} J_i K_i d_i^{1.47} \left\{ (13 + 0.01 H_g)^{0.3} + 3 \right\} \times 10^6$		
	Propeller turbine and governor:	$= 0.125 n^{0.96} J_i K_i d_i^{1.47} (1.17 H_g^{0.12} + 4) \times 10^6$		
	Pelton/Turgo turbine and governor:	$= 3.47 n^{0.96} \left(\frac{MW}{H_g^{0.5}} \right)^{0.44} \times 10^6$ where $\frac{MW}{H_g^{0.5}} > 0.4$		
		$= 5.34 n^{0.96} \left(\frac{MW}{H_g^{0.5}} \right)^{0.91} \times 10^6$ where $\frac{MW}{H_g^{0.5}} \leq 0.4$		
	Cross-flow turbine and governor:	Cost of Pelton/Turgo $\times 0.5$		
Installation of energy equipment (Eq.#5)	$= 0.15 (Eq.\#4)$			
Access road (Eq.#6)	$= 0.025 T A^2 l_a^{0.9} \times 10^6$			
Transmission line (Eq.#7)	$= 0.0011 D P l_r^{0.95} V \times 10^6$			

BASIC COSTING FORMULAE			
ITEM	SMALL	MINI	MICRO
Substation, and transformer (Eq.#8)	$= (0.0025 n^{0.95} + 0.002 (n+1)) \times \left(\frac{MW}{H_g} \right)^{0.9} \times 1^{0.3} \times 10^6$		
Installation of substation and transformer (Eq.#9)	$= 0.15 \text{ (Eq.#8)}$		
Civil works (Eq.#10)	$= 3.54 n^{-0.04} C R$ $\times \left(\frac{MW}{H_g} \right)^{0.82}$ $\times (1+0.01 I_b)$ $\times \left(1+0.005 \frac{I_d}{H_g} \right)$ $\times 10^6$	$= 1.97 n^{-0.04} C R$ $\times \left(\frac{MW}{H_g} \right)^{0.82}$ $\times (1+0.01 I_b)$ $\times \left(1+0.005 \frac{I_d}{H_g} \right)$ $\times 10^6$	$= 1.97 n^{-0.04} C$ $\times \left(\frac{MW}{H_g} \right)^{0.82}$ $\times \left(1+0.005 \frac{I_d}{H_g} \right)$ $\times 10^6$
Penstock (Eq.#11)	$= 20 n_p^{0.95} W^{0.88}$ <p>where:</p> $W = (24.7 d_p l_p t_{ane})$ <p>where:</p> $d_p = \left(\frac{Q_d}{n_p} \right)^{0.43} \frac{1}{H_g^{0.14}}$ $t_i = d_p^{1.3} + 6$ $t_b = 0.0375 d_p H_g$ $t_{ane} = 0.5(t_i + t_b) \quad \text{if } t_b \geq t_i$ $t_{ane} = t_i \quad \text{if } t_b < t_i$		



BASIC PARAMETERS			
ITEM	SMALL	MINI	MICRO
Installation of penstock (Eq.#12)		$= 5 W^{0.88}$	
Canal (Eq.#13)		$= 20 \times \left[(1.5 + 0.01 S_s^{1.5}) Q_d I_{cs} \right]^{0.9}$ <p>(for soil conditions)</p> $+$ $= 100 \times \left[(1.5 + 0.016 S_r^2) Q_d I_{cr} \right]^{0.9}$ <p>(for rock conditions)</p>	
Tunnel (Eq.#14)	$= 400 R_v^{0.88} + 4000 C_v^{0.88}$ <p>where:</p> $R_v = 0.185 I_t^{1.375} \left(\frac{Q_d^2}{k H_g} \right)^{0.375}$ $C_v = 0.306 R_v T_c$		N/A
Miscellaneous (Eq.#15)	$= 0.25 i Q_d^{0.35}$ $\times 1.1 \sum (Eq.\#2) \text{ to } (Eq.\#14)$ $+ 0.1 \sum (Eq.\#2) \text{ to } (Eq.\#14)$		$= 0.17 i$ $\times 1.1 \sum (Eq.\#2) \text{ to } (Eq.\#14)$ $+ 0.1 \sum (Eq.\#2) \text{ to } (Eq.\#14)$
Initial Costs – Total (Formula Method)		$= \sum (Eq.\#1) \text{ to } (Eq.\#15)$	



CANADIAN AND NON-CANADIAN PROJECTS - APPLICATION OF "F," "B," AND "K" FACTORS					
Costing Category	Canadian Projects		Non-Canadian Projects		
	Apply "F" Factor	% Local Component	Apply "F" to Local	Apply "B" to Local	Apply "K" to Foreign
Feasibility study (Eq.#1)		15%			
Development (Eq.#2)		50%			
Engineering (Eq.#3)	Yes	40%	Yes		Yes
Energy equipment (Eq.#4)		0%			Yes
Installation of energy equipment (Eq.#5)	Yes	100%	Yes	Yes	
Access (Eq.#6)	Yes	100%	Yes	Yes	
Transmission line (Eq.#7)	Yes	60% if V < 69 40% if V ≥ 69	Yes	Yes	Yes
Substation and transformer (Eq.#8)		0%			Yes
Installation of substation and transformer (Eq.#9)	Yes	100%	Yes	Yes	
Civil works (Eq.#10)	Yes	85%	Yes	Yes	Yes
Penstock (Eq.#11)		0%			Yes
Installation of penstock (Eq.#12)	Yes	100%	Yes	Yes	
Canal (Eq.#13)	Yes	100%	Yes	Yes	
Tunnel (Eq.#14)	Yes	85%	Yes	Yes	Yes
Miscellaneous (Eq.#15)					





RETScreen FORMULA COSTING METHOD COST CATEGORIES	
Cost Category	Formula(e) number(s)
Feasibility Study	1
Development	2
Engineering	3
Energy Equipment	4
<i>Balance of plant</i>	
Access road	6
Transmission line	7
Substation and transformer	8
Penstock	11
Canal	13
Tunnel	14
Civil works (other)	5 + 9 + 10 + 12
<i>Subtotal</i>	
Miscellaneous	15



